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[54]	METHOD AND APPARATUS FOR
	CONSOLIDATING SOILS AND IMPROVING
	THE BASE BEARING FOR FOUNDATION
	STRUCTURES

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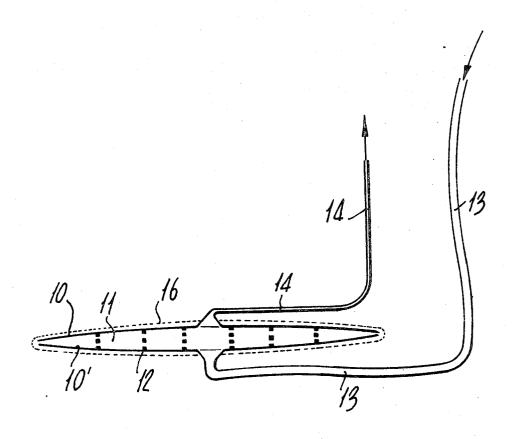
[56]	References Cited				
	UNITE	STATES PATENTS			
3,099,911	8/1963	Turzillo	61/35		
3,269,126	8/1966	Freeman, Jr	61/36 R		
3,345,824	10/1967	Turzillo	61/35		

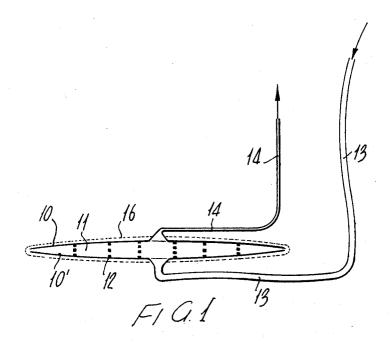
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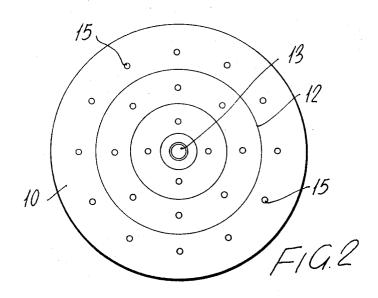
[57] ABSTRACT

A method for consolidating soils and foundation structures. Under the zone to be consolidated, a hollow impermeable rigid wall pressure cell or chamber is inserted, this cell communicating with the outside by means of an injection conduit and a vent conduit, then over said chamber or cell erecting the foundation structures (piles, walls, etc.), next injecting through the injection conduit a pressure binder mixture within the chamber previously introduced into the soil, then closing the vent conduit and continuing to inject pressure binder mixture until destroying the impermeability of said chamber and enabling a tamping or stuffing up of the neighbouring soil, then closing also the injection conduit and maintaining it at closed condition until the binder mixter injected into the soil has set, and finally washing or scavenging in site the pressure cell, whereupon the injection operation of the pressure binder mixture into the cell can be repeated also for many times by following the same method and through the same device above described.

7 Claims, 2 Drawing Figures







METHOD AND APPARATUS FOR CONSOLIDATING SOILS AND IMPROVING THE BASE BEARING FOR FOUNDATION STRUCTURES

Site cast concrete piles are known, the load bearing 5 capability of which is generally committed to the side friction soil resistance against the pile surface and base resistance on the soil bearing. Either would depend on the mechanical characteristics of the soil, as naturally such.

Evidently, where this base soil should be subjected to precompression, such as to increase the consistency thereof, the pile usable load bearing capability would be increased. A similar effect could be provided, where

Therefore, the device according to the present invention aims to provide at the base of a pile or similar structure, as site cast, a distributed pressure and pile base soil tamping and, subordinately, a soil grouting, 20 such that a strain is exerted both against the pile base (or similar structure) and neighbouring soil. The device eseentially comprises a hollow impermeable resistant shell (pressure cell) which is introduced into the soil prior to casting and arranged at the pile base. The shell 25 wall is made watertight and impermeable to prevent the internal cavity from being filled with cement liquid as the pile or similar structure is cast.

Upon cast completion, said shell is pressurized by injecting therein a binding mixture (either of a cement or 30 cially available instruments, it is also possible to obtain chemical type), that is capable of setting and hardening after some time.

Among its features, the device according to the invention is characterized in that the impermeable shell wall, or even the shell alone, will break up as a result 35 of the expansion caused by the binding mixture pressure, thus allowing the pressure liquid to diffuse within the soil and impregnate the same. Thus, the soil grouting effect will add to the mechanical tamping effect.

A possible device, such as described, is diagrammatically shown in the accompanying drawing in which:

FIG. 1 is a vertical section of the device; and FIG. 2 is a plan view of the device.

The inventive device comprises two plates 10 and 10' facing each other so as to provide an inner space 11 45 which is maintained by spacing means 12. An injection conduit 13 terminates in said cavity 11 and a vent conduit 14 branches therefrom. The device walls or plates 10, 10' are provided with a set of through holes 15 and the whole device body is enclosed by an impermeable shell 16. For example, this shell can comprise one or more layers of jute cloth or the like, externally coated with an impermeable plastic sheet. This serves the purpose of facilitating through the cloth thickness the ejection of water forming the injection binder mixture, whereby the latter may exhibit a higher density and

The device is placed on the soil at the cast base or foot (pile, wall, etc.) whereupon the casting for said overhanging structure (pile, wall, etc.) is effected. When the overhanging cement casting has set and consolidated, the device cavity (which has remained free of contamination owing to the impermeable shell 16) is filled with binding mixtures through the injection 65 hole 13. Air exits from the vent tube 14. Then, by closing said vent tube 14 and pressurizing the injected liquid, the latter will cause the whole assembly to expand,

that is plates 10, 10' will separate, the impermeable shell 16 will be filled with pressure liquid and break up at a plurality of locations, particularly towards the soil, and the liquid binding mixture will exit, thus impregnating and tamping the soil. When deeming that a convenient operation stage has been attained, for example when the soil has absorbed a sufficient amount of cement binder for its consolidation, the injection of binding mixture is shut off, the injection tube 13 is tightly 10 sealed and maintained at a constant pressure, such as by means of a gate or other system, until said pressure liquid has set. Whereupon, washing or scavenging can be effected for the whole assembly comprising the cell and the two injection and vent tubes, so that a second possible, by grouting the soil adjacent the pile base or 15 injection operation at a higher pressure can be re-

The device pressurization can be repeated also for many times at indefinite intervals. For example and particularly in loose soils, such a repetition would enable to recover or balance any sinkings a pile might undergo in time.

1 The use of the above device will allow to experimentally obtain the values of the side friction strength for a pile. Thus, by gradually loading the base cell 10, 10', the diagram can be obtained for the pile stresses of raising, similarly as an ordinary loading test. Thus, the average side strength for side friction against the pile can be obtained.

2. By inserting along the pile trunk suitable commerthrough the concrete stress condition the various side strength values at the various levels; therefore, instead of the average value of side friction, the value can be obtained as corresponding to the various layers. In any case, by suitably increasing the pressure within the base cell 10, 10' to sufficiently high values, the total load limit rate, by side friction, for the entire pile can be obtained.

3. Where the pile is of a sufficient length, or sufficiently bounded at the top or head end, the soil strength values for base load bearing can be obtained, and in the case the limit load still for base load bearing, beyond which the soil would collapse.

4. As stated at points (2) and (3), by means of the pressure cell 10, 10' it is thus possible to effect an actual pile loading test, from which such interest values can be obtained as useful load, limit load, sinkings, etc.

5. Should the pile be provided along its trunk with the instruments as mentioned at point (2), the good provision and continuity for the casting can be controlled. Thus, when pressurizing cell 10, 10' and should, for example, the pile be interrupted at a section, such instruments would detect stress values only at the trunk below the interruption, while above the interruption the values would be reduced or be zero depending on the discontinuity magnitude. It should be pointed out that at the loading tests mentioned above, the pressure cell 10, 10' substantially acts as a jack.

However, the invention importance essentially resides in a substantial unlimited cell expansion, when the stroke of an ordinary jack is necessarily limited.

It should be also specified that the sinking of a pile which has been previously pressurized through the base cell 10, 10' according to the present invention will be at any rate lower than that of an ordinary pile, and this due to both soil tamping and soil chamber laterally of the pile trunk.

6. A further application of the pressure cell 10, 10' is to be found in the injection field. By inserting a cell of the above type into an injection hole of a convenient diameter, and by tamping at the top, repeated injections can be carried out, which in addition to mixture 5 diffusing, also attain the purpose of soil tamping.

What I claim is:

1. A method for consolidating soil and improving the bearing base for a foundation structure, comprising the steps of (a) introducing beneath the soil surface a hol- 10 low enclosed cell member having an injection conduit member and a vent conduit member establishing communication between the interior of said cell member and the atmosphere above the soil surface and said cell having an impermeable, rupturable cover surrounding 15 it, (b) erecting a foundation member in a position above said cell member, (c) injecting into said cell member through the injection conduit member a binder mixture under increased pressure sufficient to rupture said impermeable covering and to cause said 20 binder mixture to penetrate the soil neighboring said cell member, (d) while maintaining said increased pressure, causing said binder mixture injected into the soil to set, (e) injecting into said cell member through the injection member a washing liquid thereby washing all 25 comprises two opposing apertured convex-concave unset binder mixture from the interior of said cell member and from said injection conduit and vent conduit members, and (f) repeating at least once the steps (c), (d) and (e) set forth above.

ing the step of providing means responsive to said injec-

tion of binder mixture under pressure for measuring physical forces exerted on said foundation structure by the soil as a result of said injection of binder mixture.

3. The method according to claim 2, wherein said measuring means is incorporated in the foundation member.

4. The method according to claim 1, wherein said foundation member is a concrete structure.

5. A device suitable for introduction into the soil to effect consolidation thereof and improve the bearing support for foundation structures, comprising a hollow enclosed cell member comprised of two opposing apertured plates separated from one another and mutually supported by a plurality of spacing means, an injection conduit member communicating with the interior chamber of said cell, a vent conduit member communicating with the interior chamber of said cell, and a covering layer surrounding said cell, said cover being impermeable but capable of being ruptured by means of a binder mixture forced under pressure into the hollow interior of said cell through said injection conduit member.

6. The device according to claim 5, wherein said cell plates separated from one another and mutually supported by a plurality of spacing means.

7. The device according to claim 5, wherein said covering comprises at least one layer of jute linen and a 2. The method according to claim 1, further compris- 30 second exterior layer of impermeable plastic material.

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